



Quinte Health

Making our communities healthier. Together.

BELLEVILLE GENERAL HOSPITAL

**Phase 2 Work Required for Generator &
Fuel System Replacement Project**

Contract No. CAP24-037-RFP02

APPENDIX E

IFT Structural Package

D01) GENERAL

D01-1 GENERAL INFORMATION

- THE INFORMATION PRESENTED ON THESE DRAWINGS HAS BEEN DESIGNED AND ANALYZED IN ACCORDANCE WITH THE 2024 ONTARIO BUILDING CODE & 2020 NATIONAL BUILDING CODE OF CANADA. CONSTRUCTION IS TO BE PERFORMED IN ACCORDANCE WITH THIS AND ALL OTHER APPLICABLE CODES.

■ 1.1

CONCRETE STRUCTURE DESIGNED IN ACCORDANCE WITH CSA A23.3-19

■ 1.2

STEEL STRUCTURE DESIGNED IN ACCORDANCE WITH CSA S16-19
- ALL WORK MUST COMPLY WITH THE PROVISIONS OF THE 2024 ONTARIO BUILDING CODE, OCCUPATIONAL HEALTH & SAFETY ACT, MUNICIPAL BYLAWS AND BEST CODE PRACTICES. THE CONTRACTOR MUST CONSIDER THAT CONSTRUCTION WORK IS DEPENDENT ON SITE CONDITIONS.
- ALL DIMENSIONS ARE IN MILLIMETRES AND ELEVATIONS IN METRES UNLESS INDICATED OTHERWISE.
- CONTRACTOR IS TO VERIFY / COORDINATE ALL DIMENSIONS / PENETRATIONS WITH MECHANICAL AND ELECTRICAL DRAWINGS PRIOR TO CONSTRUCTION. REPORT INCONSISTENCIES BEFORE PROCEEDING WITH WORK. ANY OPENINGS NOT INDICATED ON STRUCTURAL DRAWINGS ARE TO BE APPROVED BY STRUCTURAL ENGINEER IN WRITING PRIOR TO CONSTRUCTION.
- CAD / REVIT VERSIONS OF THE STRUCTURAL DRAWINGS SHALL BE MADE AVAILABLE TO THE CONTRACTOR UPON THE COMPLETION OF A RELEASE FORM INDEMNIFYING THE CONSULTANT FROM ANY ERRORS OR OMISSIONS ASSOCIATED WITH THE CAD / REVIT FILES.
- SEISMIC RESTRAINT OF MECH AND ELECT ELEMENTS NOT NOTED ON THE DRAWINGS ARE THE RESPONSIBILITY OF THE CONTRACTOR'S ENGINEER. RESTRAINT DETAILS ARE TO BE DEVELOPED IN ACCORDANCE WITH THE 2024 ONTARIO BUILDING CODE. CONTRACTOR'S ENGINEER IS RESPONSIBLE FOR THE DESIGN AND DETAILING OF SEISMIC RESTRAINTS AND ISOLATIONS AS REQUIRED BY SPECIFICATIONS INCLUDING THE VERIFICATION THAT THE EXISTING / NEW STRUCTURE IS CAPABLE OF SAFELY SUPPORTING THE IMPOSED LOADS IN ACCORDANCE WITH THE 2024 ONTARIO BUILDING CODE. NO ELEMENTS MAY BE CONSTRUCTED WITHOUT WRITTEN CONFIRMATION OF THESE CONDITIONS BY CONTRACTOR'S ENGINEER.
- NO FOUNDATION ELEMENTS ARE TO BE CONSTRUCTED UNTIL WRITTEN APPROVAL OF THE BEARING SURFACES AND PRESSURES IS PROVIDED BY A GEOTECHNICAL ENGINEER THROUGH ON-SITE INVESTIGATION. FAILURE TO COMPLETE THIS WORK COULD RESULT IN THE REMOVAL / REINSTATEMENT OF ANY / ALL FOUNDATION ELEMENTS AT CONTRACTOR'S OWN COST.
- CONTRACTOR TO PROVIDE PRE-ENGINEERED SHORING AS REQUIRED TO ACCOMMODATE THE CONTRACTOR'S CONSTRUCTION ACTIVITIES AND TO PREVENT DAMAGE TO ANY ADJACENT PROPERTY. ALL CONSTRUCTION ACTIVITIES TO BE LIMITED TO THE LIMITS OF THE CONSTRUCTION SITE AND ALL DAMAGE TO EXISTING PROPERTIES MUST BE REINSTATED.
- DO NOT SCALE DIMENSIONS DIRECTLY FROM THESE DRAWINGS OR ELECTRONIC FILES.
- SPECIFIC NOTES AND DETAILS FOUND ON THESE DRAWINGS SHALL TAKE PRECEDENCE OVER GENERAL NOTES AND TYPICAL DETAILS.

D01-2 DEMOLITION

- CONTRACTOR TO SCAN FOR ALL SERVICES AND CALL FOR LOCATES PRIOR TO DEMOLITION. ALL LOCATES AND SCANS TO BE PAID FOR BY CONTRACTOR. ENSURE TO PROVIDE LOCATE REPORT TO CLIENT.
- CONTRACTOR SHALL EXERCISE EXTREME CAUTION WHILE DEMOLISHING EXISTING STRUCTURE AS TO NOT DAMAGE THOSE PORTIONS OF THE STRUCTURE THAT ARE TO REMAIN.
- CONTRACTOR SHALL MAKE GOOD, TO THE SATISFACTION OF THE CONSULTANT, ANY DAMAGE THAT OCCURS DUE TO THE DEMOLITION PHASE.
- ALL DEMOLISHED MATERIALS MUST BE DISPOSED OF OFF-SITE AT THE END OF EACH WORKING DAY.

D01-3 SHOP DRAWINGS

- SUBMIT SHOP DRAWINGS FOR ALL STRUCTURAL WORK AND ANY WORK AFFECTING THE STRUCTURE TO THE STRUCTURAL CONSULTANT. OBTAIN ENGINEER'S APPROVAL BEFORE PROCEEDING WITH THE FABRICATION.
- EACH OF THE FOLLOWING SHOP DRAWINGS MUST BEAR THE SIGNATURE AND STAMP OF A QUALIFIED PROFESSIONAL ENGINEER REGISTERED IN THE PROVINCE OF ONTARIO (PLUS OTHER DRAWINGS AS NOTED).

a) SEISMIC ANCHORAGE OF ALL MECHANICAL UNITS AND TANKS
- SHOP DRAWINGS MUST BE REVIEWED AND STAMPED REVIEWED BY THE CONTRACTOR BEFORE ISSUING TO THE ENGINEER. SHOP DRAWINGS NOT STAMPED BY THE CONTRACTOR WILL BE REJECTED. ANY DELAYS IN THE CONSTRUCTION SCHEDULE DUE TO NONCOMPLIANCE WITH THIS REQUIREMENT SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR.
- SHOP DRAWINGS ARE REVIEWED FOR CONFORMANCE WITH THE GENERAL DESIGN CONCEPT. THIS REVIEW DOES NOT IMPLY APPROVAL OF THE DETAILED DESIGN OR QUANTITIES DESCRIBED IN THE SHOP DRAWINGS. THE RESPONSIBILITY FOR THE QUANTITIES AND DETAILED DESIGN OF THE MATERIALS AND COMPONENTS AS REQUIRED TO PROVIDE THE COMPLETE AND SATISFACTORY JOB DESCRIBED IN THE DESIGN DOCUMENTS REMAINS WITH THE CONTRACTOR.

D02-0 EXCAVATION, FOUNDATIONS, AND BACKFILL

D02-1 FOOTINGS:

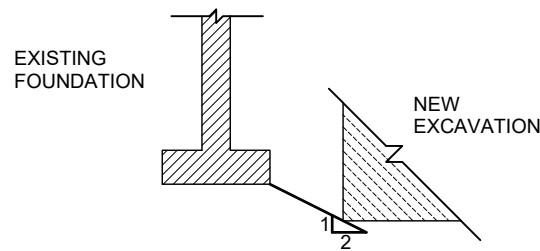
ALL FOOTINGS TO BEAR ON UNDISTURBED NATIVE MATERIAL WITH MINIMUM ALLOWABLE BEARING STRENGTH OF 100KPa. CONTRACTOR TO RETAIN THE SERVICES OF A GEOTECHNICAL ENGINEER TO PROVIDE WRITTEN CONFIRMATION OF BEARING CAPACITY FOR REVIEW AND APPROVAL PRIOR PLACEMENT OF CONCRETE.

D02-2 EXCAVATION, FOUNDATIONS AND BACKFILL:

- PRIOR TO ANY EXCAVATION, VERIFY LOCATION OF EXISTING SERVICES AND TAKE ALL NECESSARY MEASURES TO MAINTAIN SERVICES WHERE REQUIRED. NOTIFY OWNER AND ENGINEER IF ANY SERVICES NOT SHOWN ON PLAN OR OTHERWISE EXPECTED ARE ENCOUNTERED. DO NOT PROCEED FURTHER UNTIL DIRECTED.
- CARE MUST BE TAKEN TO AVOID UNDERMINING EXISTING BUILDING FOUNDATIONS OR UNDERGROUND SERVICES.
- PROTECT SUB-GRADE FROM FREEZING AND FROST ACTION AT ALL TIMES DURING CONSTRUCTION.
- FOOTINGS MUST BEAR ON APPROVED BEARING SURFACES.
- BACKFILL TO WITHIN 300mm OF UNDERSIDE OF SLAB WITH GRANULAR 'B' TYPE II IN LAYERS UP TO 300 THICK, COMPACTED TO MINIMUM 98% SPMD. UNLESS NOTED OTHERWISE.
- FINAL 300mm UNDER SLAB TO BE GRANULAR 'A' COMPACTED TO MINIMUM 98% SPMD, UNLESS NOTED OTHERWISE.
- RE-USE OF EXCAVATED GRANULAR MATERIAL IS NOT ACCEPTABLE.
- CONTRACTOR TO RETAIN THE SERVICES OF A GEOTECHNICAL ENGINEER TO PROVIDE WRITTEN CONFIRMATION OF THE COMPACTED GRANULAR MATERIAL WAS INSTALL AS SPECIFIED FOR REVIEW AND APPROVAL.

D02-3 PROTECT LATERAL STABILITY OF BEARING STRATA UNLESS NOTED:

UNLESS OTHERWISE OUTLINED IN CONTRACTOR'S GEOTECHNICAL ENGINEER REPORT DO NOT EXCAVATE BELOW A LINE EXTENDING DOWNWARD FROM ANY BEARING STRATA AT A SLOPE OF 1 VERTICAL TO 2 HORIZONTAL. ADJUST FOOTING AND TRENCH ELEVATIONS TO MEET THIS REQUIREMENT (SEE DIAGRAM).



D03) CONCRETE

D03-1 CONCRETE COVER (CLEAR TO REINFORCING):

US FOOTINGS & SLAB ON GRADE (AGAINST SOIL)
SLAB ON GRADE (SIDES & TOP)

75mm
50mm

D03-2 CONCRETE REINFORCING STEEL:

- REINFORCING TO BE GRADE 400R DEFORMED BARS TO CSA-G30.18 AND GRADE 400W WHERE BARS ARE TO BE WELDED.
- SPACING OF BARS SHALL BE APPROXIMATELY UNIFORM WITHIN THE CONCRETE ELEMENTS. DO NOT ELIMINATE OR DISPLACE REINFORCEMENT TO ACCOMMODATE HARDWARE. OBTAIN APPROVAL OF ALL MODIFICATIONS FROM CONSULTANT BEFORE THE PLACING OF CONCRETE.
- ALL LAPS AND EMBEDMENT OF DOWELS SHALL BE 40 BAR DIAMETERS, BUT NOT LESS THAN 600mm IF NOT SPECIFIED OTHERWISE. WIRE MESH LAPS SHALL BE 150mm MINIMUM.
- TYPICAL REBAR REQUIREMENTS:

BAR DESIGNATION:

METRIC: 10-15M MEANS 10 BARS, SIZE 15M.
- PROVIDE STANDARD HOOKS WHERE TOP AND BOTTOM BARS TERMINATE AT EDGES.
- ALL DOWELS TO BE IN LINE WITH THE VERTICAL OR HORIZONTAL STEEL WITHIN THE STRUCTURAL ELEMENT UNLESS OTHERWISE NOTED
- DETAIL, FABRICATE AND PLACE ALL REINFORCEMENT IN CONFORMITY TO CURRENT MANUAL OF STANDARD PRACTICE FOR DETAILING REINFORCED CONCRETE STRUCTURES ACI 315, CSA CAN3-A23.3 - LATEST EDITION AND REINFORCING STEEL MANUAL OF STANDARD PRACTICE BY RSIO.
- ALL REINFORCING STEEL TO BE CHAIRED AND SECURELY TIED IN PLACE USING STANDARD TIES AND CHAIRS TO THE REQUIRED COVER FOR EXPOSED CONCRETE. CHAIRS AND BOLTERS TO BE PLASTIC TIPPED OR STAINLESS STEEL.
- SUBMIT REINFORCING SHOP DRAWINGS DETAILING FOR STRUCTURAL ENGINEER REVIEW PRIOR TO FABRICATION.

D03-3 CONCRETE MIXES

PROPORTION NORMAL DENSITY CONCRETE IN ACCORDANCE WITH CAN/CSA-A23.1, TO GIVE THE FOLLOWING QUALITY FOR ALL CONCRETE AS INDICATED.

LOCATION	28 DAY STRENGTH	SUMP (SEE NOTE 6)	CLASS OF EXPOSURE
SLAB ON GRADE (EXTERIOR)	35 MPa		
SLAB ON GRADE (INTERIOR)	25 MPa		C-1

* OBTAIN THESE SLUMPS WITH AID OF SPECIFIED WATER REDUCING AGENT.
* NOTE: ALL CONCRETE EXPOSED TO EXTERIOR CONDITIONS TO HAVE MINIMUM 6% AIR ENTRAINMENT.

READY-MIXED CONCRETE AND CONCRETE PROPORTIONS SHALL BE IN ACCORDANCE WITH CSA A23.1, CLAUSE 12 AND AS FOLLOWS:

- MINIMUM ALLOWABLE COMPRESSIVE STRENGTH SHALL BE 30 MPa (4400psi) AT 28 DAYS OF AGE, UNLESS OTHERWISE NOTED OR SHOWN.
- IF BLENDED NORMAL PORTLAND CEMENT/CEMENTITIOUS HYDRAULIC SLAG IS USED EXCEPT FOR FLOOR MIXES, SLAG CONTENT SHALL NOT BE MORE THAN 25% OF TOTAL MASS OF CEMENT. TOTAL VOLUME OF CEMENT IN CONCRETE FLOOR MIXES SHALL BE 100% NORMAL PORTLAND CEMENT.
- PROVIDE CERTIFICATION THAT MIX PROPORTIONS SELECTED WILL PRODUCE CONCRETE OR SPECIFIED YIELD AND QUALITY AND THAT STRENGTH WILL COMPLY WITH CAN/CSA-A23.1-M06.
- USE OF CALCIUM CHLORIDE NOT PERMITTED.
- DO NOT CHANGE CONCRETE MIX WITHOUT PRIOR APPROVAL OF CONSULTANT. SHOULD CHANGE IN MATERIAL SOURCE BE PROPOSED, NEW MIX DESIGN TO BE APPROVED BY CONSULTANT.
- UNLESS SPECIFIED, SLUMP TO BE AS REQUIRED TO FACILITATE CONSTRUCTION TECHNIQUES, PLACEMENT, AND REBAR CONGESTION.
- CONCRETE FINISHING: BROOM FINISH

D03-4 FIELD QUALITY CONTROL

- INSPECTION AND TESTING OF CONCRETE AND CONCRETE MATERIALS WILL BE CARRIED OUT BY A TESTING LABORATORY RETAINED AND PAID FOR BY THE CONTRACTOR IN ACCORDANCE WITH CAN/CSA-A23.1.
- AIR ENTRAINMENT TEST AND SLUMP TEST MADE FROM SAME BATCH OF CONCRETE FROM WHICH TEST CYLINDERS ARE MADE.
- TESTS WILL BE MADE IN ACCORDANCE WITH CSA A23.2.
- INSPECTION COMPANY'S REPORTS OF TESTS WILL BE FORWARDED TO THE CONSULTANT AND CONTRACTOR WITH AN OPINION OR REASON FOR ANY ABNORMALITIES NOTED THEREON.
- COOPERATE WITH AND ASSIST INSPECTION COMPANY'S PERSONNEL DURING INSPECTION AND TESTS.
- REMOVE DEFECTIVE MATERIALS AND COMPLETED WORK WHICH FAILS TESTS AND REPLACE AS DIRECTED BY THE CONSULTANT.
- WHERE WORK OR MATERIALS FAIL TO MEET STRENGTH REQUIREMENTS AS INDICATED BY TEST RESULTS, PAY COSTS OF ADDITIONAL INSPECTION AND TESTING REQUIRED FOR NEW REPLACEMENT WORK OR MATERIALS.
- NON-DESTRUCTIVE METHODS FOR TESTING CONCRETE SHALL BE IN ACCORDANCE WITH CAN/CSA-A23.2.
- INSPECTION OR TESTING BY THE CONSULTANT WILL NOT AUGMENT OR REPLACE CONTRACTOR QUALITY CONTROL NOR RELIEVE HIM OF HIS CONTRACTUAL RESPONSIBILITY.

D03-5 CONCRETE COLD WEATHER PROTECTION

- CONTRACTOR IS RESPONSIBLE TO PROVIDE TEMPORARY HEAT, INSULATION MATERIALS OR OTHER MEANS AS REQUIRE TO PROTECT THE MASONRY BLOCKS, GROUT, CONCRETE AND GRANULAR MATERIALS FROM FREEZING DURING THE WORK. IN ACCORDANCE WITH CSA-A23.1.
- PROTECT SUB-GRADE FROM FREEZING AND FROST ACTION AT ALL TIMES DURING CONSTRUCTION. DO NOT PLACE CONCRETE ON FROZEN GROUND.
- CONTRACTOR TO PROVIDE PROPOSED METHOD OF PROTECTION FOR OUR REVIEW PRIOR TO COMMENCING THE WORK.
- CURE CONCRETE TO CSA A23.1/A23.2. TAKE APPROPRIATE PRECAUTIONS FOR COLD WEATHER WORK AS PER CSA A23.1/A23.2.

D03-6 CONCRETE WARM WEATHER PROTECTION

- CARRY OUT HOT WEATHER CONCRETING IN ACCORDANCE WITH CSA A23.1.
- PROTECT CONCRETE FROM EFFECT OF HOT OR DRYING WEATHER CONDITIONS. PROTECT FORMS AND REINFORCING FROM THE DIRECT RAYS OF THE SUN, OR COOL BY FOGGING AND EVAPORATION.
- CURE CONCRETE TO CSA A23.1/A23.2. TAKE APPROPRIATE PRECAUTIONS FOR WARM WEATHER WORK AS PER CSA A23.1/A23.2.

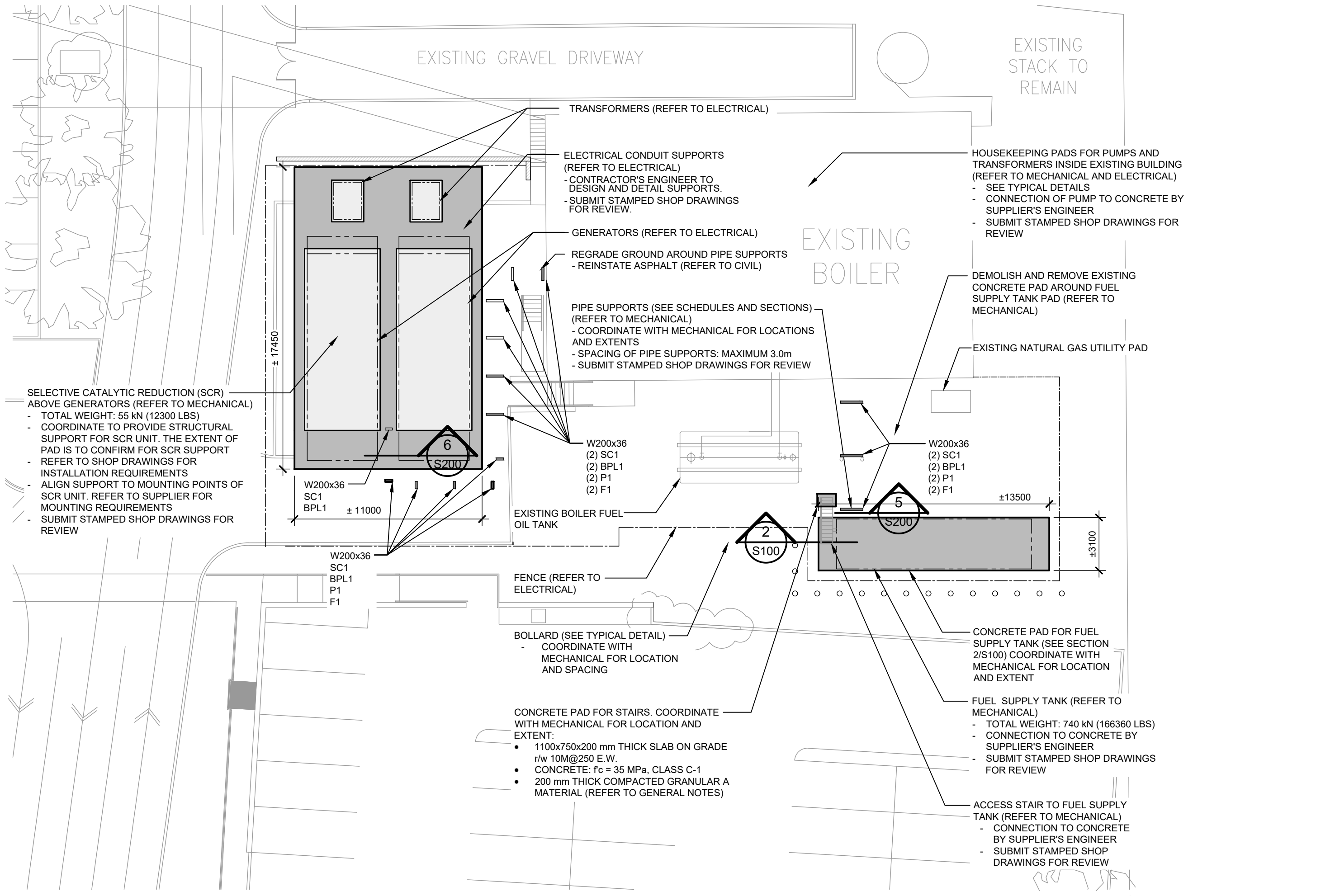
D04) STEEL

D04-1 STRUCTURAL STEEL:

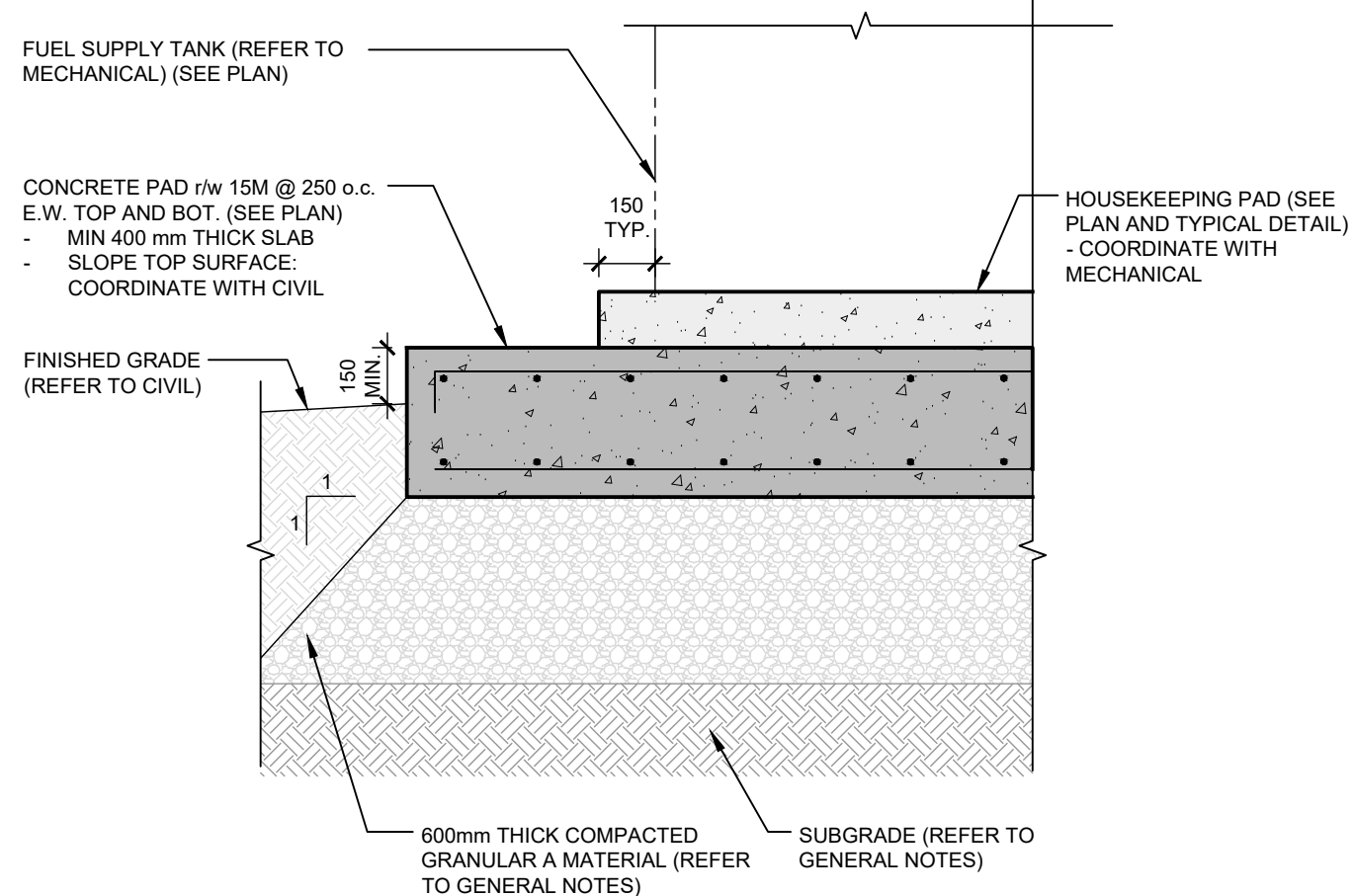
STRUCTURAL STEEL SHALL COMPLY WITH CSA S16-19 UNLESS OTHERWISE NOTED.

ITEM	APPLICABLE SPECIFICATION (UNLESS OTHERWISE NOTED)
ROLLED SECTIONS	G40 21 - 350W
ROLLED C & L SHAPES	G40 21 - 300W
HSS (TUBE) SECTIONS	G40 21 - 350W (CLASS C)
CONNECTION BOLTS	A325 (BEARING TYPE)
ANCHOR BOLTS	A307 (UNLESS OTHERWISE NOTED IN BASEPLATE SCHEDULE)
BRACE FRAME/BEARING PLATES	G40 21 - 350W

- ALL STEEL WORK SHALL BE GIVEN ONE COAT OF APPROVED PRIMER.
- FIELD AND SHOP CONNECTIONS SHALL BE WELDED OR HIGH TENSILE BOLTED (ASTM STANDARD A325).
- WELDING SHALL CONFORM TO LATEST CSA SPECIFICATION W59 AND BE UNDERTAKEN BY A FABRICATOR APPROVED BY THE CANADIAN WELDING BUREAU TO THE REQUIREMENTS OF CSA SPECIFICATION W47.1.
- ALL EXPOSED WELDS SHALL BE CONTINUOUS AND BE GROUND SMOOTH.
- ALL EXTERIOR EXPOSED STRUCTURAL STEEL SHALL BE GALVANIZED OR PAINTED WITH APPROVED RUST INHIBITIVE PAINT.
- STRUCTURAL STEEL MEMBERS SHALL NOT BE SPLICED UNLESS APPROVED BY THE STRUCTURAL ENGINEER IN WRITING.
- WHERE STRUCTURAL STEEL MEMBERS SPECIFIED ON THE STRUCTURAL DRAWINGS ARE UNAVAILABLE TO THE CONTRACTOR, THE STRUCTURAL STEEL CONTRACTOR SHALL PROVIDE MEMBERS HAVING ALL SECTION PROPERTIES EQUAL TO OR BETTER THAN THAT OF THE SPECIFIED MEMBERS AT NO ADDITIONAL COST. CONTACT ENGINEER FOR ACCEPTANCE OF ANY AND ALL SUBSTITUTIONS.



1 PLAN: CONCRETE PADS
SCALE 1:200



2 DETAIL: CONCRETE PAD
SCALE 1:20

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7	APR. 24/26	ISSUED FOR TENDER - PHASE 2	VP
6	FEB. 20/26	ISSUED FOR BUILDING PERMIT RESUBMISSION	KT
5	DEC. 05/25	ISSUED FOR ADDENDUM 01	KT
4	NOV. 07/25	ISSUED FOR PERMIT/TENDER	KT
3	OCT. 14/25	ISSUED FOR TENDER	KT
2	SEP. 26/25	ISSUED FOR FINAL REVIEW	EK
1	JUN. 18/25	ISSUED FOR OWNER'S REVIEW	BM
No.	DATE	REVISIONS	BY

PROJECT NORTH	STAMP



PROJECT	QUINTE HEALTH BELLEVILLE GENERAL HOSPITAL GENERATOR REPLACEMENT
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DRAWING	GENERAL NOTES
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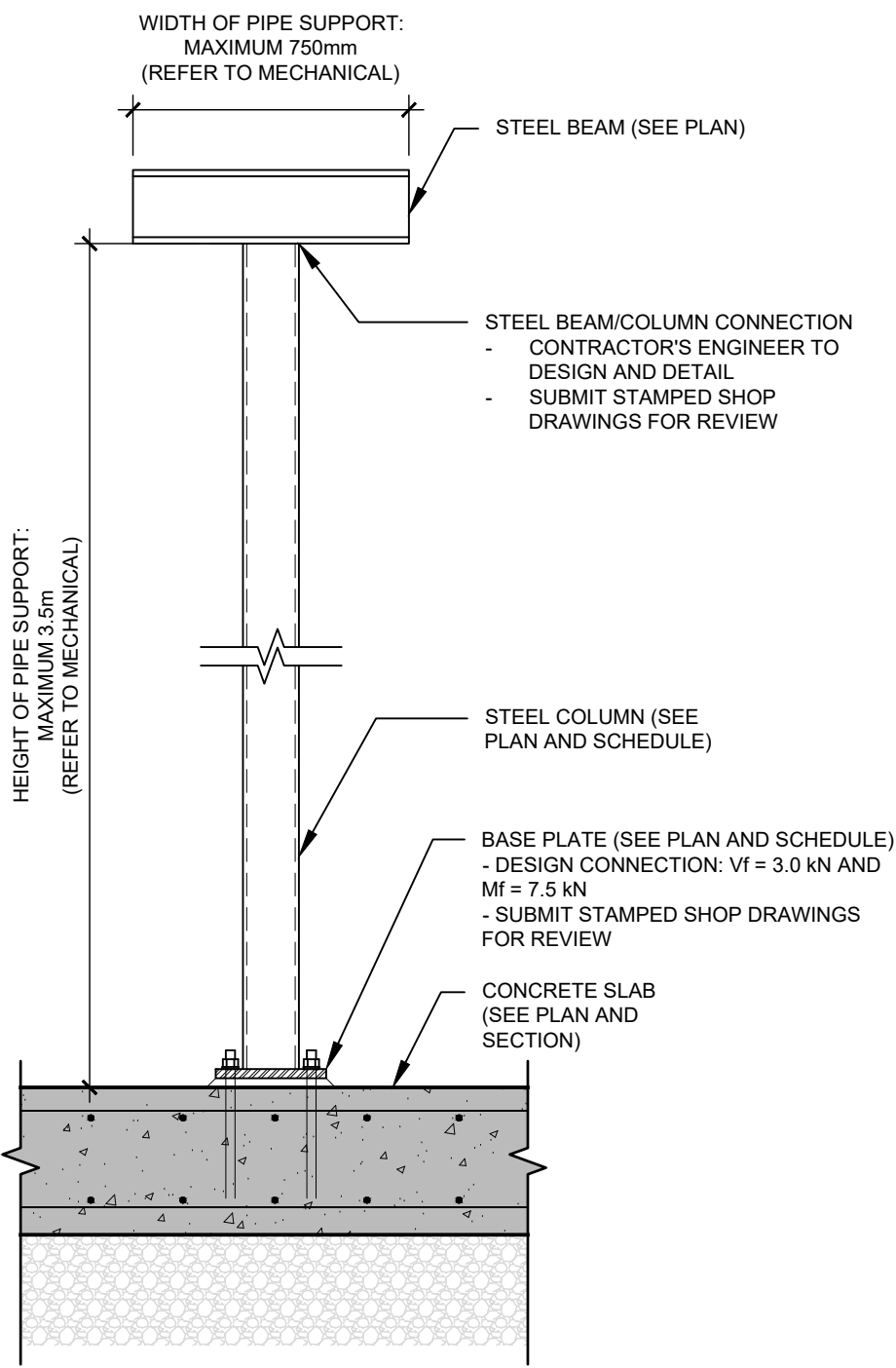
DRAWN: B. METZ	DRAWING No.	S100
DESIGNED: E. RICHER		
DATE: JUNE 2025		
SCALE: AS NOTED		
PROJECT No: 25-1058		

FOOTING SCHEDULE		
MARK	SIZE (LxWxT)	NOTES
F1	BF36 BIGFOOT	$Q_{ult} \leq 75$ KPa $Q_{ult} \leq 110$ KPa
NOTES: 1. ALL FOOTINGS TO BEAR ON SUITABLE BEARING STRATA AS APPROVED BY GEOTECHNICAL ENGINEER WITH MINIMUM BEARING PRESSURES NOTED IN TABLE. 2. PROVIDE FROST COVER TO ALL FOOTINGS EXPOSED TO EXTERIOR CONDITIONS AS APPROVED BY GEOTECHNICAL ENGINEER. 3. CONCRETE STRENGTH: $f_c = 25$ MPa, CLASS N		

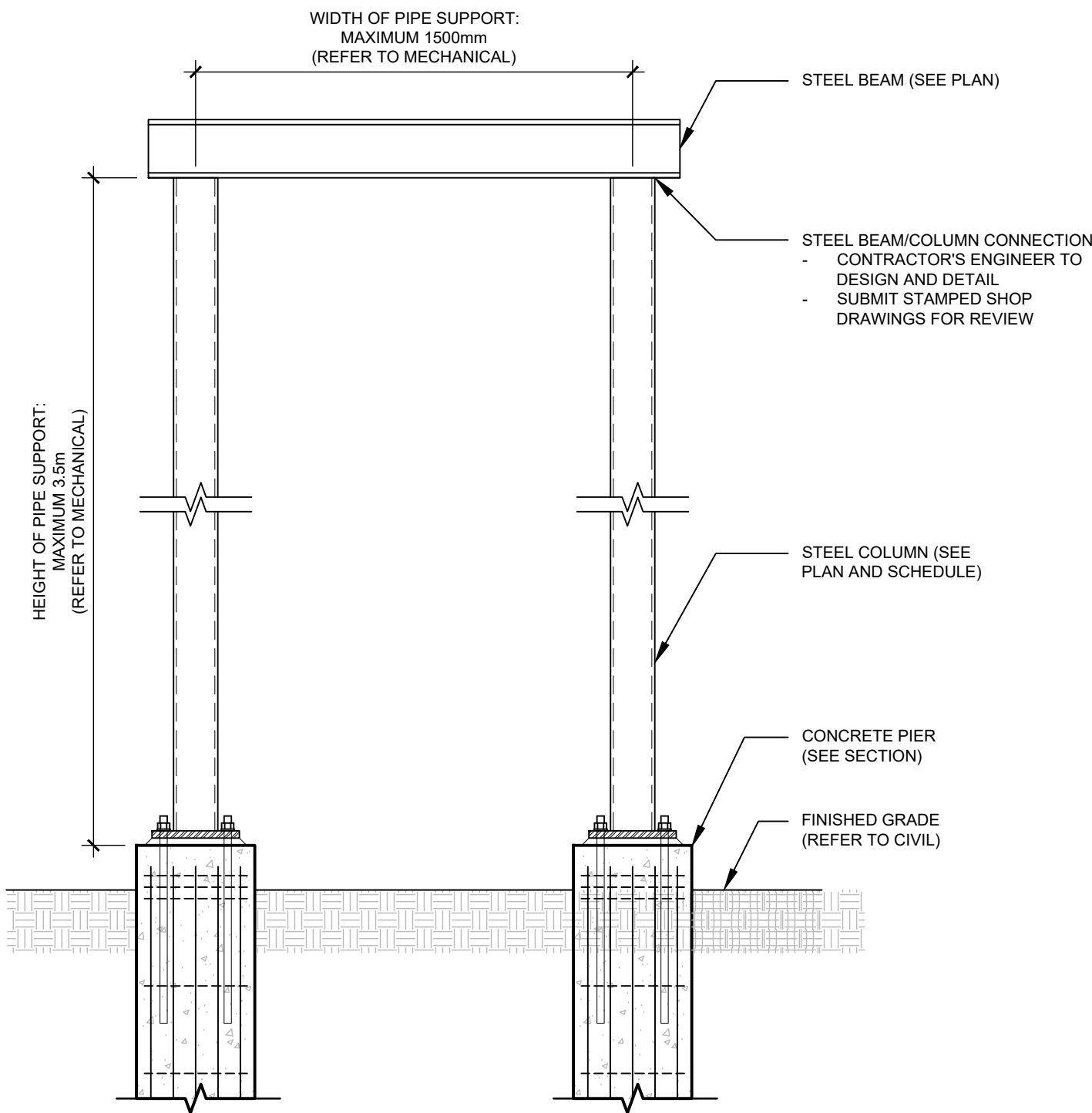
PIER SCHEDULE		
MARK	SIZE	REINFORCING
P1	457 DIA SONOTUBE	8-20M VERT. 10M TIES @ 300mm o.c. +ADDT. TIE GROUP AT TOP
NOTES: 1. TOPS OF PIERS TO BE SET +/- 150 (6") ABOVE FINISHED GRADE. REFER TO CIVIL AND SITE PLAN 2. PIERS DIMENSIONS ARE TO BE CENTRED ON SUPPORTED COLUMNS (U.N.O.). 3. VERTICAL BARS IN PIERS MAY BE LAPPED WITH COMPRESSION LAPS WHERE NO BRACE/MOMENT FRAMES ARE PRESENT. WHERE BRACE/MOMENT FRAMES ARE PRESENT VERTICAL BARS ARE TO BE EXTEND TO FULL HEIGHT ABOVE FOOTING TO TOP OF PIERS (NO LAPS ARE PERMITTED UNLESS APPROVED BY ENGINEER IN WRITING). CLASS 'B' TENSION LAPS (MIN. 1.5d) MAY BE USED WHERE HEIGHT OF PIER EXCEEDS 2400 (8'-0"). 4. CONCRETE STRENGTH: $f_c = 25$ MPa EXTERIOR OR EXTERIOR WALL: CLASS F-2 INTERIOR: CLASS N 5. PROVIDE ADDITIONAL GROUP OF TIES AT TOP 150mm(6") SPACING		

STEEL COLUMN SCHEDULE		
MARK	SIZE	NOTES
SC1	HSS 152x152x9.5	
NOTES: 1. REFER TO PLANS FOR ELEVATIONS AND ORIENTATIONS. 2. ALL HSS COLUMNS TO BE CLASS 'C' A500. 3. CONTRACTOR TO COORDINATE LENGTHS OF COLUMNS BEARING AT PIERS AND INTERIOR FOOTINGS.		

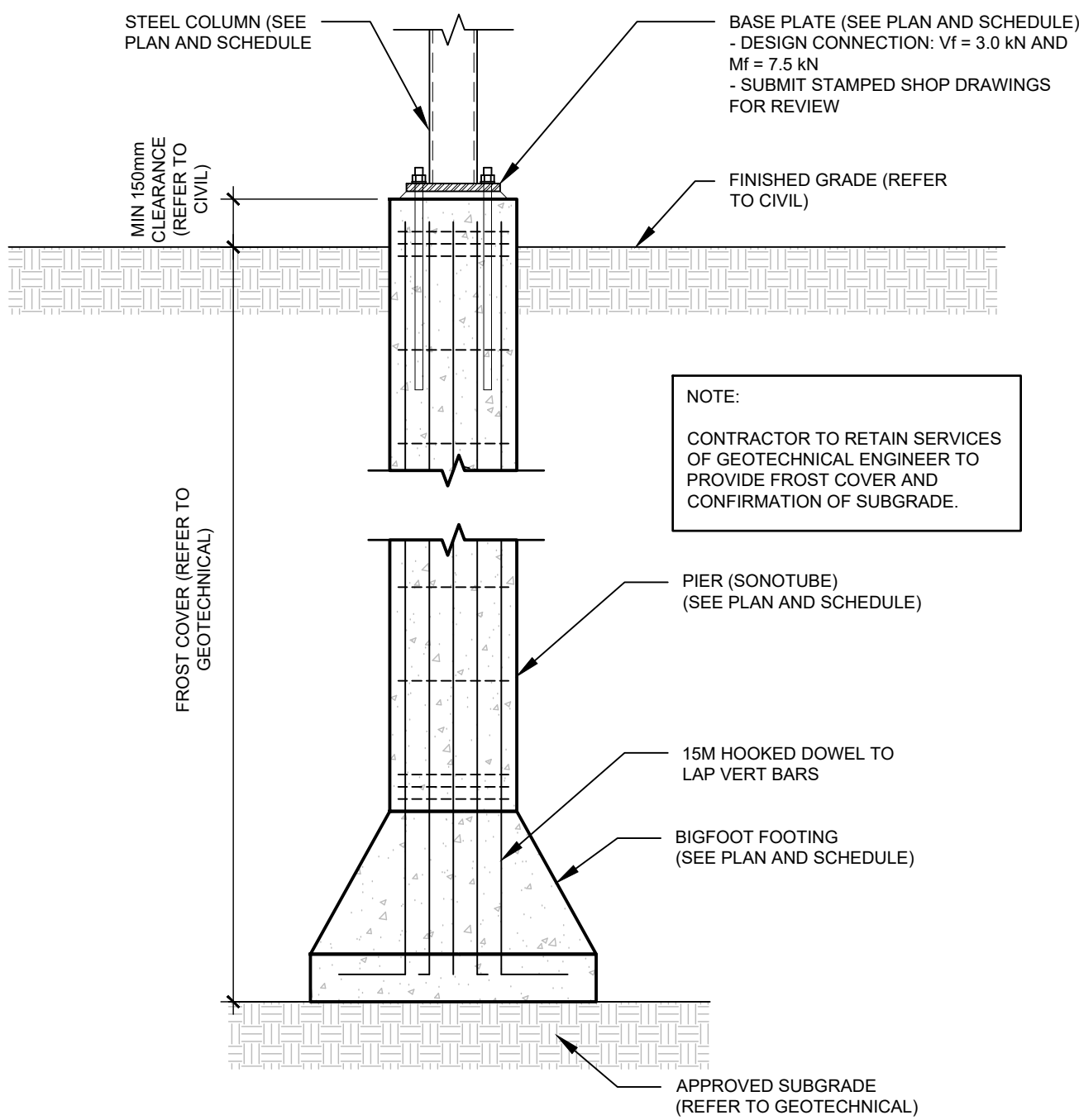
BASEPLATE SCHEDULE		
MARK	SIZE (LxWxT)	NOTES
BPL1	300x300x25	4-25mm $\varnothing$ HEADED ANCHOR RODS MIN 600mm EMBEDMENT
NOTES: 1. LENGTH OF ANCHORS IS EMBEDMENT LENGTH. FABRICATOR TO PROVIDE ADDITIONAL LENGTH AS REQUIRED FOR THREADS/TOLERANCES, ETC. 2. ALL ANCHORS TO BE CAST-IN (CONCRETE) OR GROUTED INTO PLACE (MASONRY). 3. PROVIDE MIN. 25(1") THICK GROUT BED BELOW BASE PLATE U/N. BASEPLATE ON SLAB: ANCHOR ROD TO BE EMBEDDED MINIMUM 325mm INTO CONCRETE		



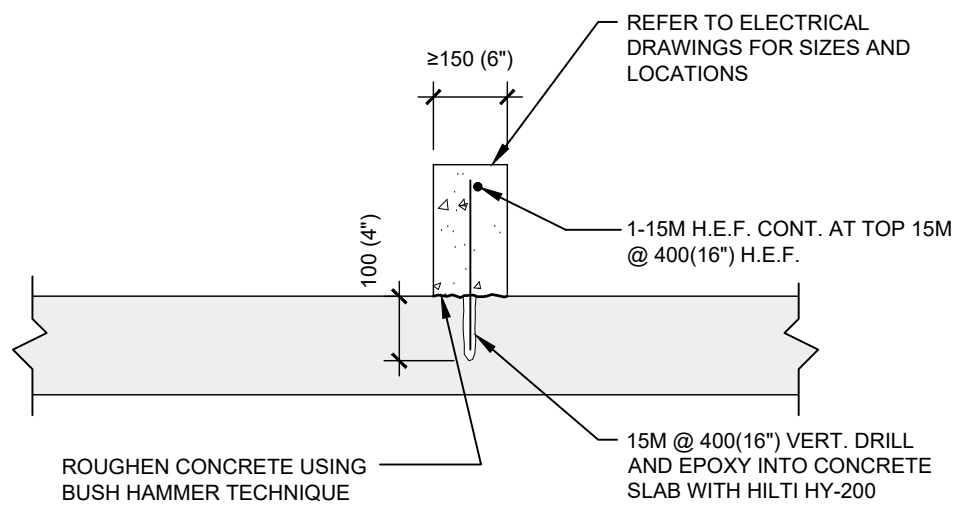
6 SECTION: PIPE SUPPORT (SINGLE COLUMN) ON SLAB ON GRADE
S200 SCALE 1:20



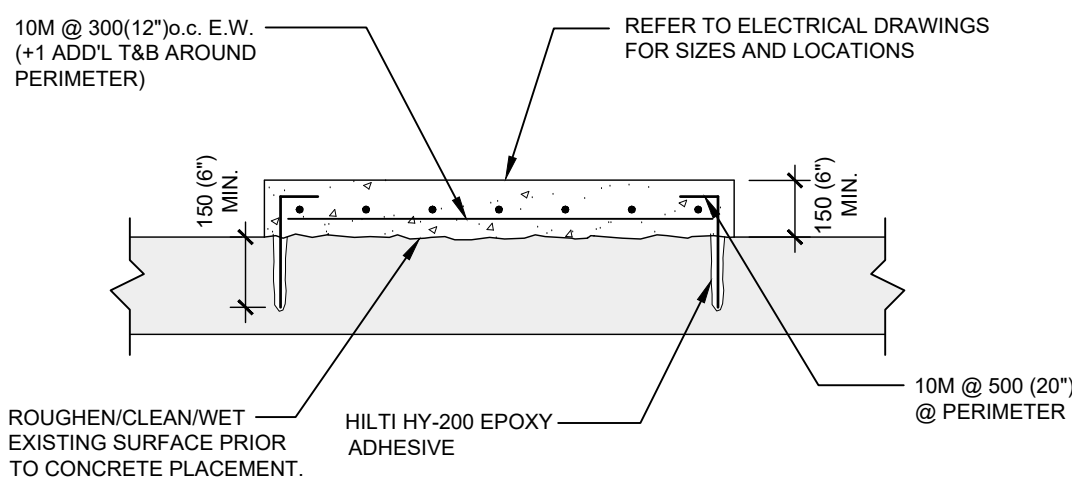
5 SECTION: PIPE SUPPORT (DOUBLE COLUMNS) ON PIER FOUNDATION
S200 SCALE 1:20



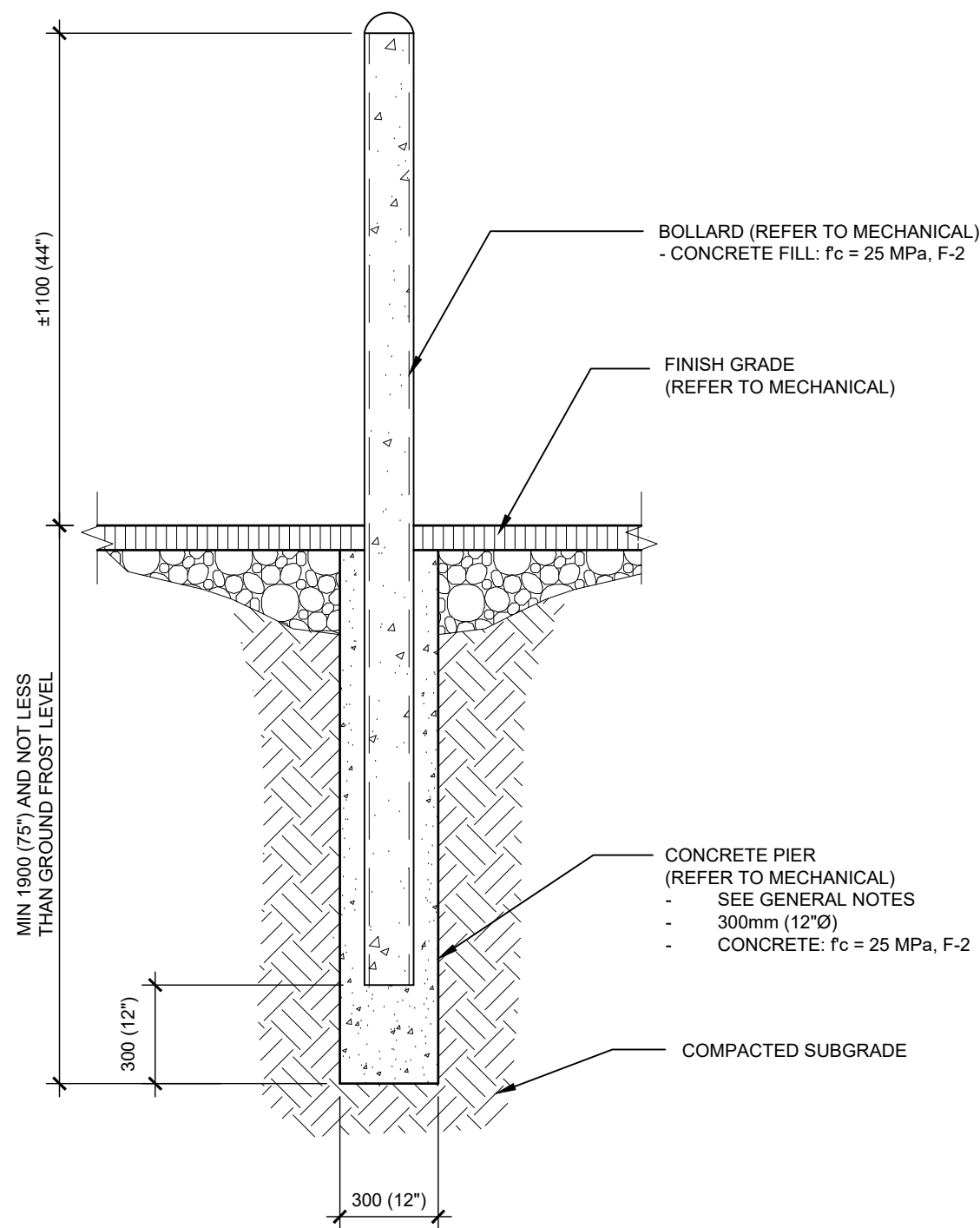
4 SECTION: PIER FOUNDATION WITH BIGFOOT FOOTING
S200 SCALE 1:20



3 DETAIL: TYPICAL MECHANICAL CURBS
S200 SCALE 1:20



2 DETAIL: TYPICAL MECHANICAL PADS
S200 SCALE 1:20



1 DETAIL: TYPICAL BOLLARD AT GRADE
S200 SCALE 1:20

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PROJECT NORTH	STAMP



PROJECT	QUINTE HEALTH BELLEVILLE GENERAL HOSPITAL GENERATOR REPLACEMENT
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DRAWING	TYPICAL DETAILS
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DRAWN: B. METZ	DRAWING No.
DESIGNED: E. RICHER	
DATE: JUNE 2025	S200
SCALE: AS NOTED	
PROJECT No: 25-1058	

PLOTTED ON: 27-Apr-2026